

Pressure Vessel and Combined Loading

MAE 314 – Solid Mechanics

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Thin-Walled Pressure Vessels



Cylindrical vessel with capped ends

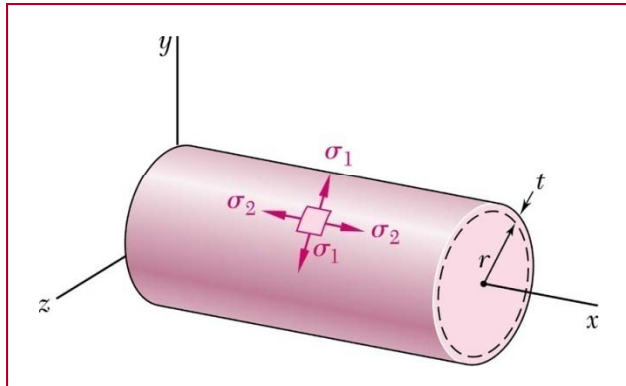


Spherical vessel

- Assumptions
 - Constant gage pressure, p = internal pressure – external pressure
 - Thickness much less than radius ($t \ll r$, $t / r < 0.1$)
 - Internal radius = r
 - Point of calculation far away from ends (St. Venant's principle)



Cylindrical Pressure Vessel

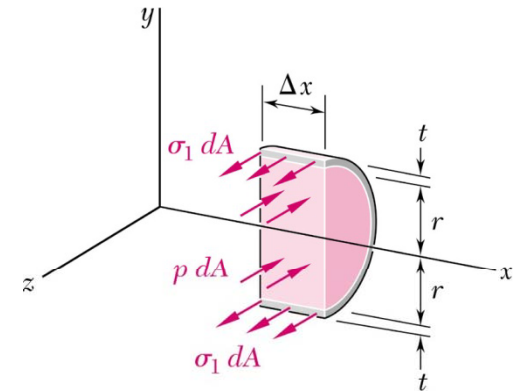


Circumferential (Hoop) Stress: σ_1

Sum forces in the vertical direction.

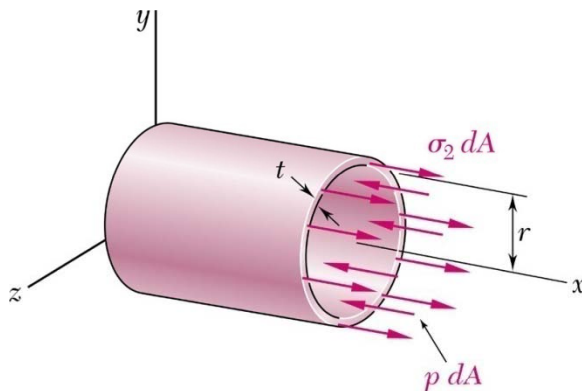
$$2\sigma_1(t\Delta x) - p(2r\Delta x) = 0$$

$$\sigma_1 = \frac{pr}{t}$$



Longitudinal stress: σ_2

Sum forces in the horizontal direction:



$$\sigma_2(2\pi r t) - p(\pi r^2) = 0$$

$$\sigma_2 = \frac{pr}{2t}$$



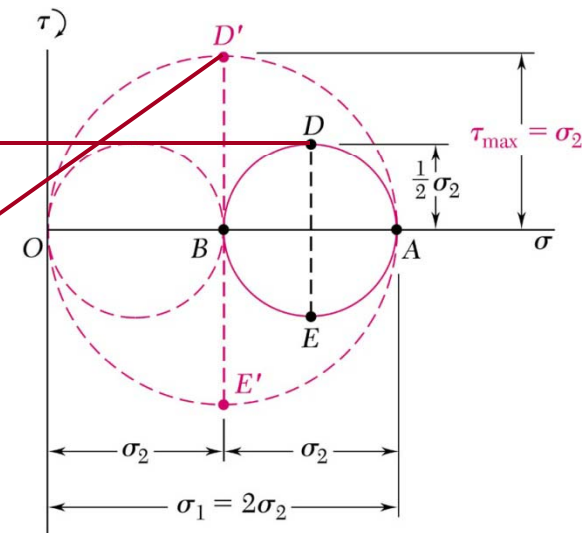
Cylindrical Pressure Vessel cont'd

- There is also a radial component of stress because the gage pressure must be balanced by a stress perpendicular to the surface.
 - $\sigma_r = p$
 - However $\sigma_r \ll \sigma_1$ and σ_2 , so we assume that $\sigma_r = 0$ and consider this a case of plane stress.
- Mohr's circle for a cylindrical pressure vessel:
 - Maximum shear stress (in-plane)

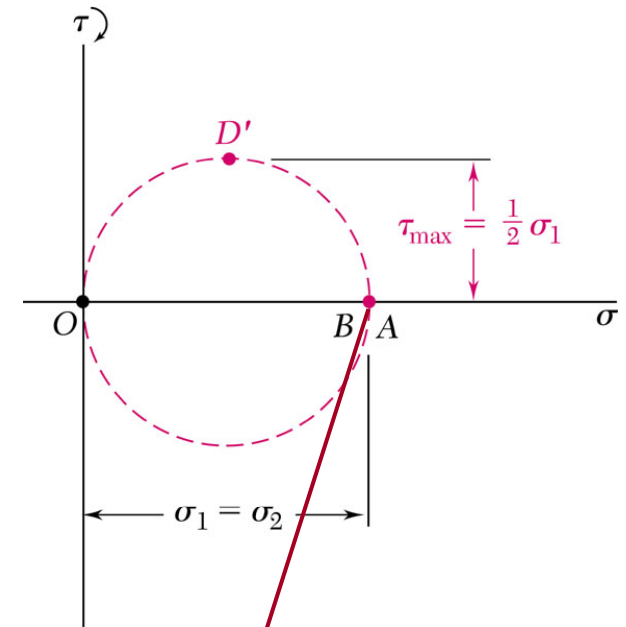
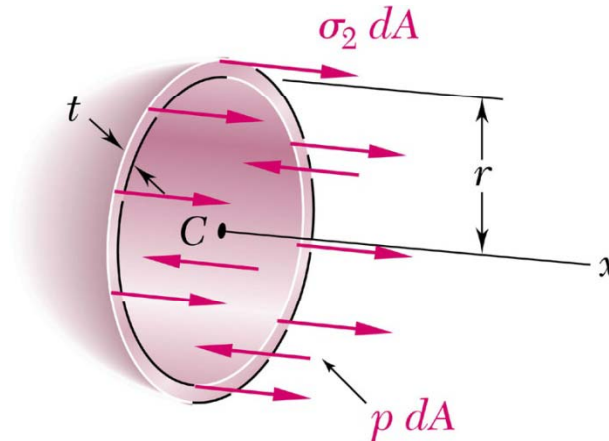
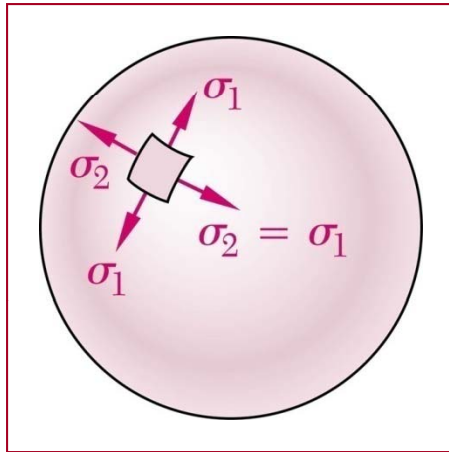
$$\tau_{\max} = \frac{\sigma_2}{2} = \frac{pr}{4t}$$

- Maximum shear stress (out-of-plane)

$$\tau_{\max} = \sigma_2 = \frac{pr}{2t}$$



Spherical Pressure Vessel



Sum forces in the horizontal direction:

$$\sigma_2(t2\pi r) - p(\pi r^2) = 0$$

$$\sigma_1 = \sigma_2 = \frac{pr}{2t}$$

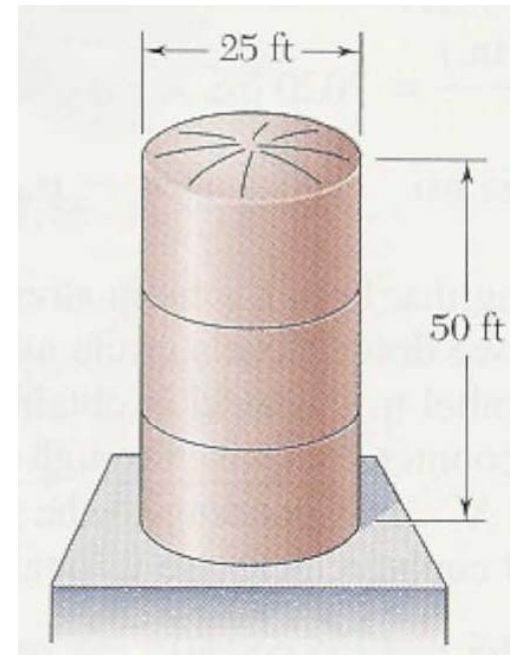
In-plane Mohr's circle is just a point

$$\tau_{\max} = \frac{\sigma_1}{2} = \frac{pr}{4t}$$



Example Problem 1

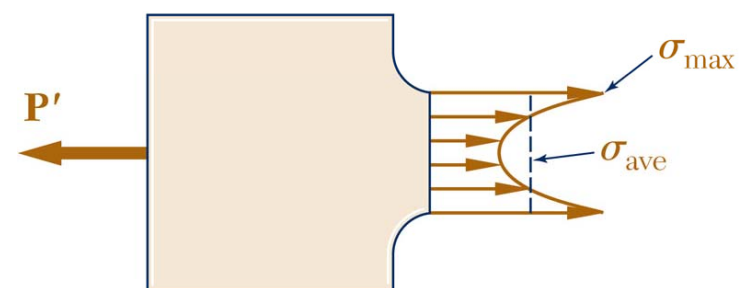
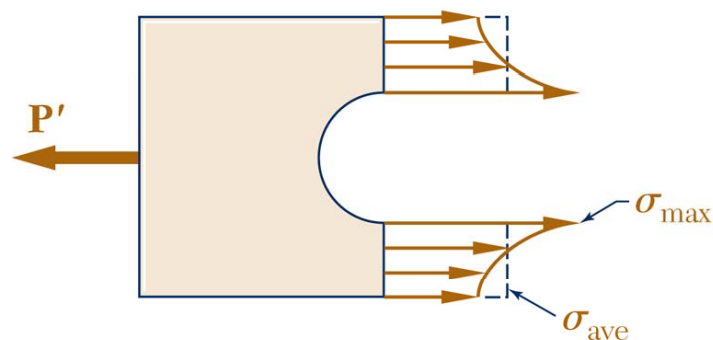
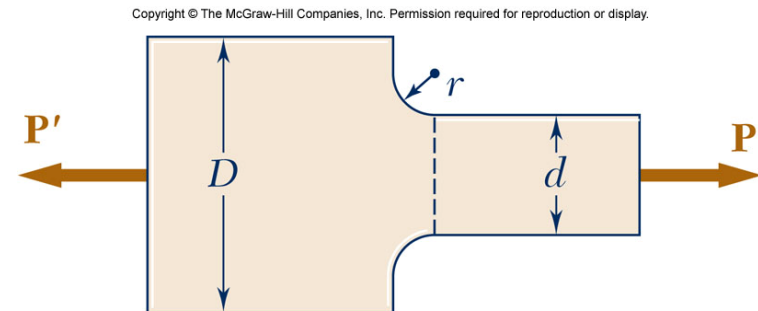
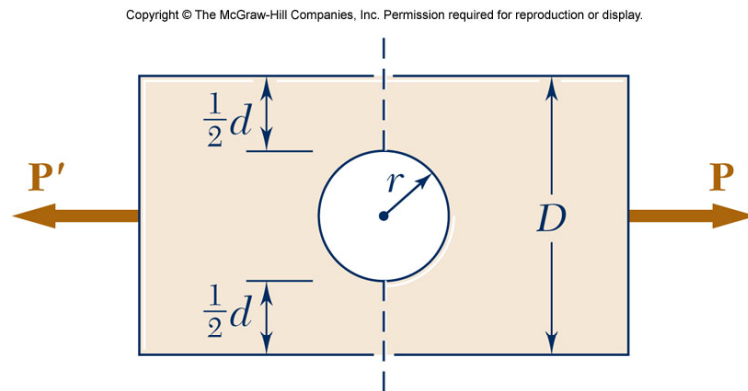
When filled to capacity, the unpressurized storage tank shown contains water to a height of 48 ft above its base. Knowing that the lower portion of the tank has a wall thickness of 0.625 in, determine the maximum normal stress and the maximum shearing stress in the tank. (Specific weight of water = 62.4 lb/ft^3)



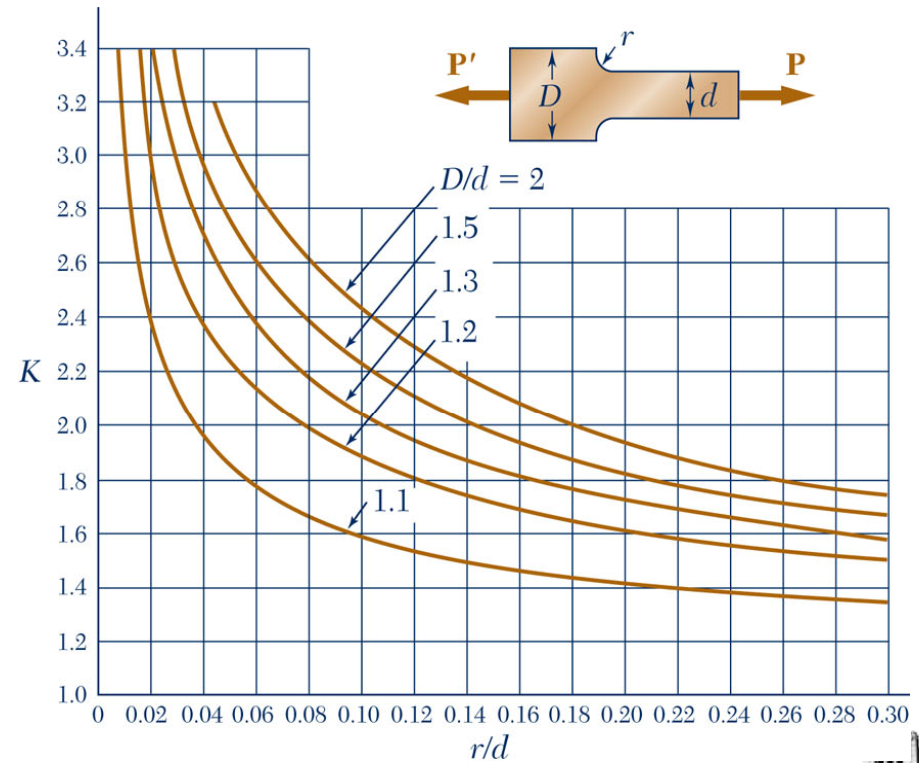
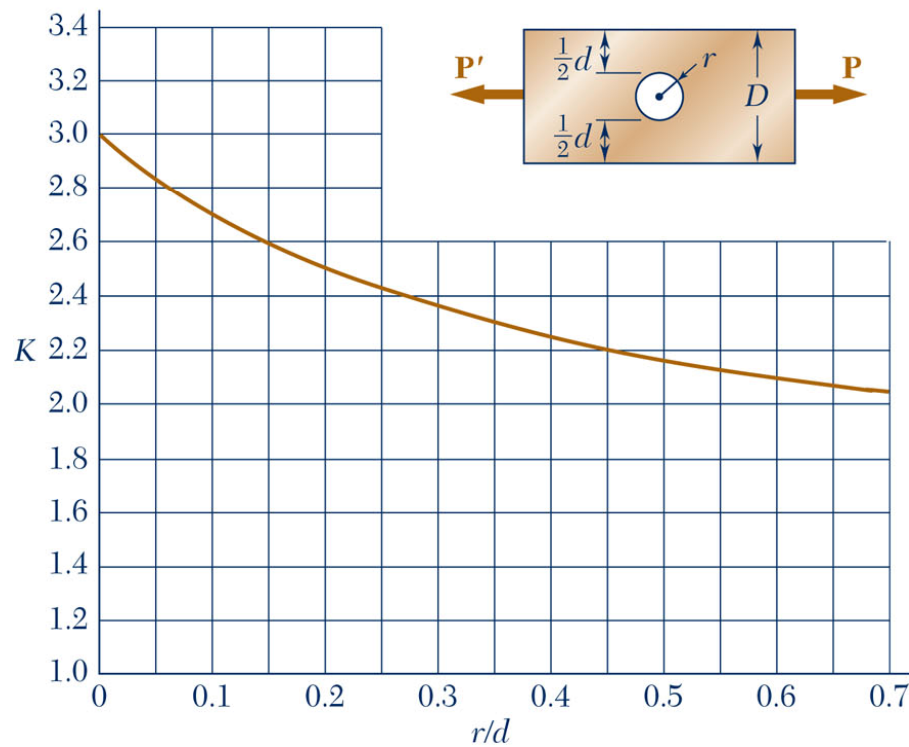


Stress Concentration

- The stresses near the points of application of concentrated loads can reach values much larger than the average value of the stress in the member.
- Stress concentration factor, $K = \sigma_{\max} / \sigma_{\text{ave}}$



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Example

- Determine the largest axial load P that can safely supported by a flat steel bar consisting of two portions, both 10 mm thick and, respectively, 40 and 60 mm wide, connected by fillets of radius $r = 8$ mm. assume an allowable normal stress of 165 MPa. (Example 2.12 in Beer's book)

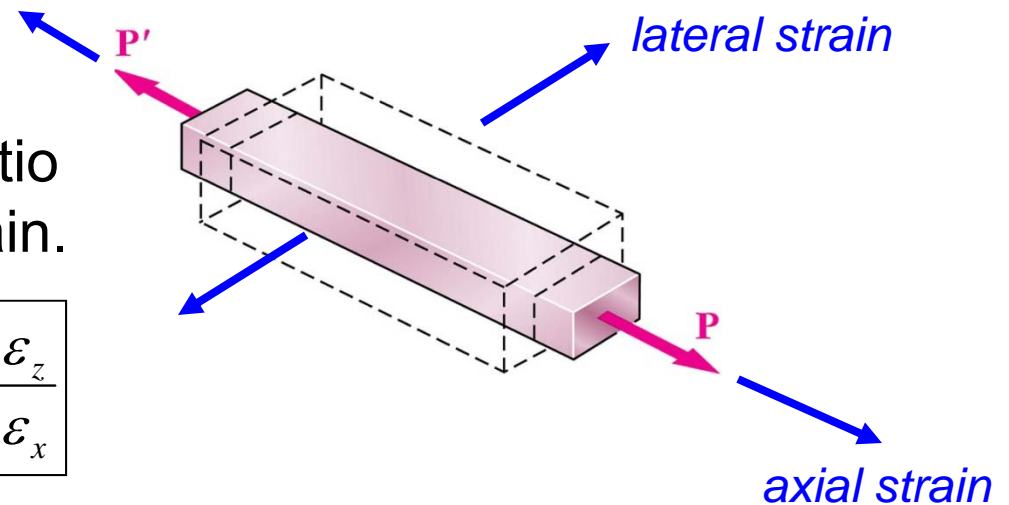


Poisson's Ratio

- When an axial force is applied to a bar, the bar not only elongates but also shortens in the other two orthogonal directions.
- Poisson's ratio (ν) is the ratio of lateral strain to axial strain.

$$\nu = - \frac{\text{lateral strain}}{\text{axial strain}} = - \frac{\epsilon_y}{\epsilon_x} = - \frac{\epsilon_z}{\epsilon_x}$$

Minus sign needed to obtain a positive value – all engineering materials have opposite signs for axial and lateral strains



- ☐ ν is a material specific property and is dimensionless.

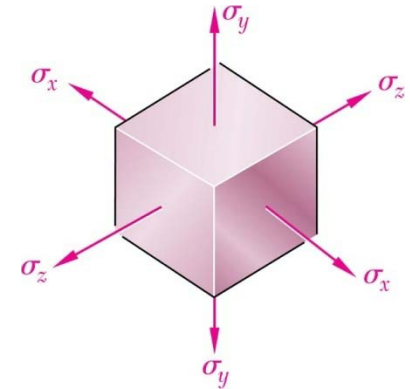


Generalized Hooke's Law

- Let's generalize Hooke's Law ($\sigma = E\varepsilon$).
 - Assumptions: linear elastic material, small deformations

$$\varepsilon_x = \frac{\sigma_x}{E} - \frac{\nu\sigma_y}{E} - \frac{\nu\sigma_z}{E} \quad \varepsilon_y = -\frac{\nu\sigma_x}{E} + \frac{\sigma_y}{E} - \frac{\nu\sigma_z}{E}$$

$$\varepsilon_z = -\frac{\nu\sigma_x}{E} - \frac{\nu\sigma_y}{E} + \frac{\sigma_z}{E}$$



- So, for the case of a homogenous isotropic bar that is axially loaded along the x-axis ($\sigma_y=0$ and $\sigma_z=0$), we get

$$\varepsilon_x = \frac{\sigma_x}{E} \quad \varepsilon_y = \varepsilon_z = -\frac{\nu\sigma_x}{E}$$

Even though the stress in the y and z axes are zero, the strain is not!



Poisson's Ratio *cont'd*

- What are the limits on ν ? We know that $\nu > 0$.

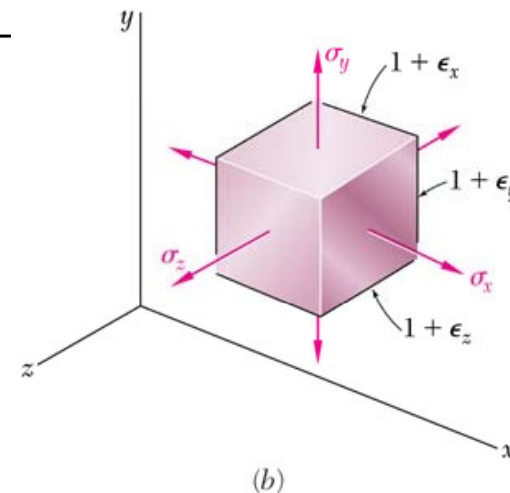
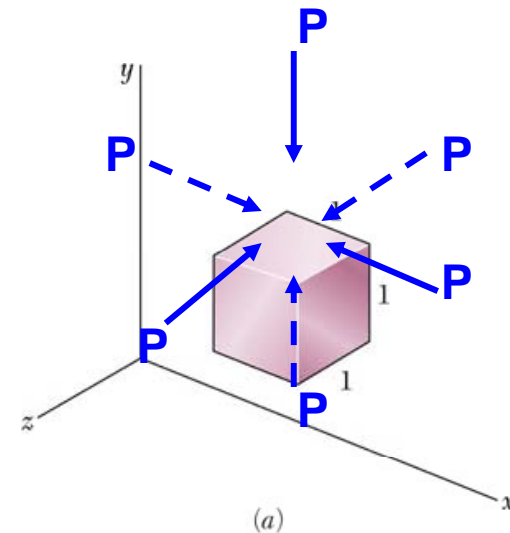
- Consider a cube with side lengths = 1
- Apply hydrostatic pressure to the cube

$$\sigma_x = \sigma_y = \sigma_z = -P$$

- Can write an expression for the change in volume of the cube

$$\begin{aligned}\Delta V &= (1 + \epsilon_x)(1 + \epsilon_y)(1 + \epsilon_z) - 1 \\ &= 1 - 1 + \epsilon_x + \epsilon_y + \epsilon_z + \cancel{\epsilon_x \epsilon_y} + \cancel{\epsilon_x \epsilon_z} + \cancel{\epsilon_y \epsilon_z} + \epsilon_x \epsilon_y \epsilon_z\end{aligned}$$

$\epsilon_x, \epsilon_y, \epsilon_z$ are very small, so
we can neglect the terms of
order ϵ^2 or ϵ^3



Poisson's Ratio *cont'd*

- ΔV simplifies to $\Delta V \cong \varepsilon_x + \varepsilon_y + \varepsilon_z$
- Plug $\sigma=P/A=P$ into our generalized equations for strain.

$$\varepsilon_x = \frac{\sigma_x}{E} - \frac{\nu\sigma_y}{E} - \frac{\nu\sigma_z}{E} = -\frac{P}{E} + \frac{\nu P}{E} + \frac{\nu P}{E} = \frac{P}{E}(2\nu - 1)$$

$$\varepsilon_y = -\frac{\nu\sigma_x}{E} + \frac{\sigma_y}{E} - \frac{\nu\sigma_z}{E} = \frac{\nu P}{E} - \frac{P}{E} + \frac{\nu P}{E} = \frac{P}{E}(2\nu - 1)$$

$$\varepsilon_z = -\frac{\nu\sigma_x}{E} - \frac{\nu\sigma_y}{E} + \frac{\sigma_z}{E} = \frac{\nu P}{E} + \frac{\nu P}{E} - \frac{P}{E} = \frac{P}{E}(2\nu - 1)$$

- Plug these values into the expression for ΔV .

$$\Delta V \cong \frac{3P}{E}(2\nu - 1)$$



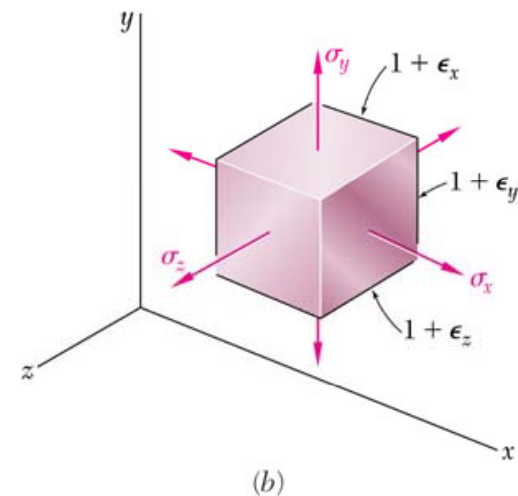
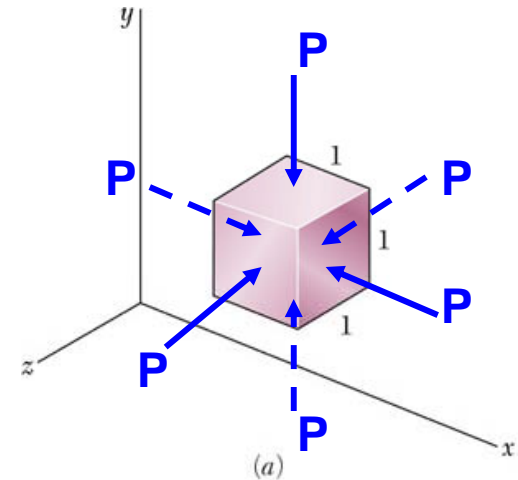
Poisson's Ratio *cont'd*

- Since the cube is compressed, we know ΔV must be less than zero.

$$\frac{3P}{E}(2\nu - 1) < 0$$

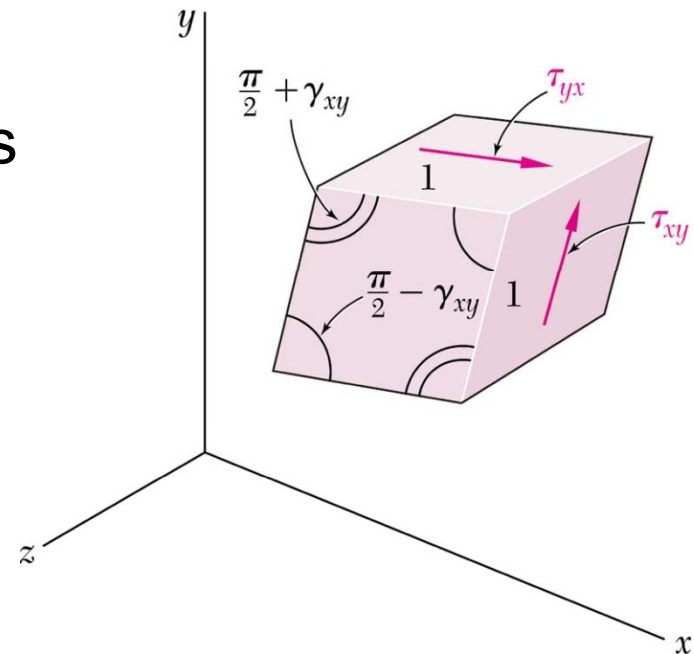
$$2\nu - 1 < 0$$

$$0 < \nu < \frac{1}{2}$$



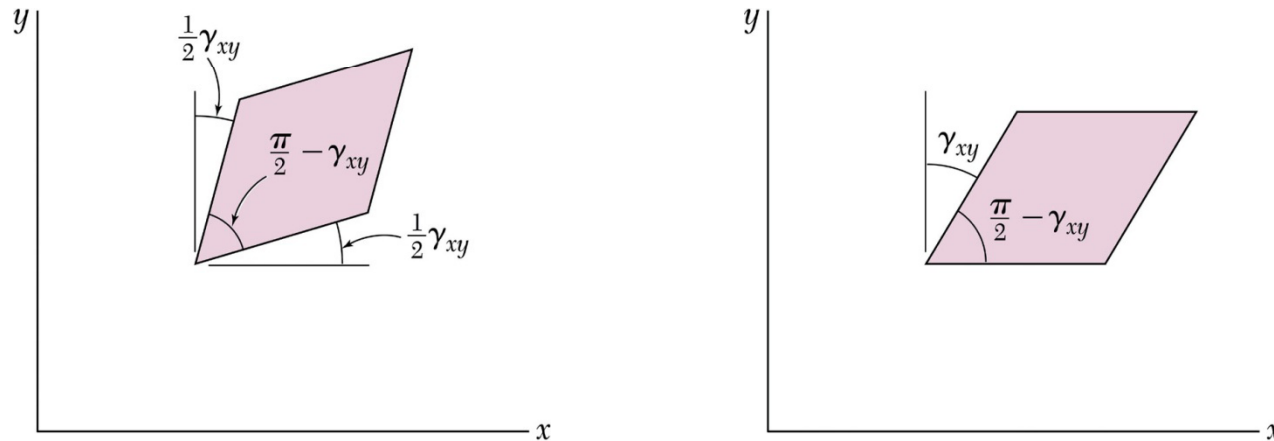
Shear Strain

- Recall that
 - Normal stresses produce a change in **volume** of the element
 - Shear stresses produce a change in **shape** of the element
- Shear strain (γ) is an angle measured in degrees or radians (dimensionless)
- Sign convention is the same as for shear stress (τ)



Shear Strain *cont'd*

- There are two equivalent ways to visualize shear strain.



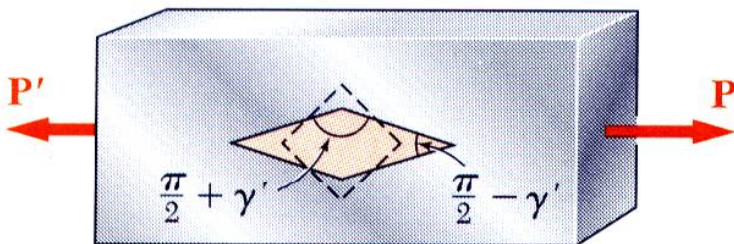
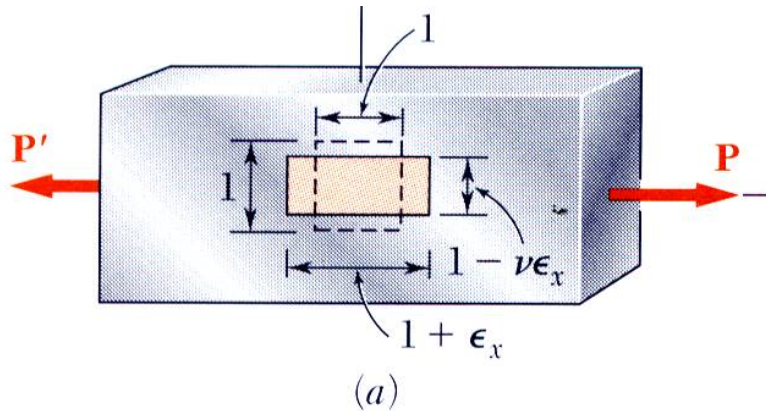
- Hooke's Law for shear stress is defined as

$$\tau_{xy} = G\gamma_{xy} \quad \tau_{xz} = G\gamma_{xz} \quad \tau_{yz} = G\gamma_{yz}$$

- G = shear modulus (or modulus of rigidity)
- G is a material specific property with the same units as E (psi or Pa).



Relation Among E , ν , and G



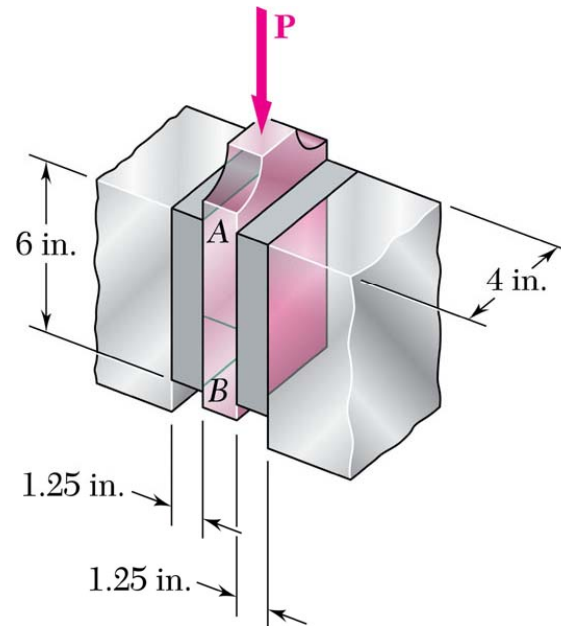
- An axially loaded slender bar will elongate in the axial direction and contract in the transverse directions.
- An initially cubic element oriented as in top figure will deform into a rectangular parallelepiped. The axial load produces a normal strain.
- If the cubic element is oriented as in the bottom figure, it will deform into a rhombus. Axial load also results in a shear strain.
- Components of normal and shear strain are related,

$$\frac{E}{2G} = (1 + \nu)$$



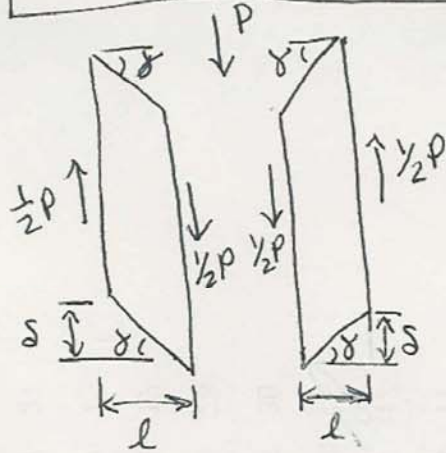
Example Problem 1

- A vibration isolation unit consists of two blocks of hard rubber bonded to a plate AB and to rigid supports as shown. Knowing that a force of magnitude $P = 6$ kips causes a deflection of $\delta = 0.0625$ in. of plate AB, determine the modulus of rigidity of the rubber used.



Example Problem 1 Solution

Lect. 6: Example Problem 1



$$P = 6 \text{ kips} \Rightarrow V = \frac{1}{2} (6 \text{ kips}) = 3 \text{ kips}$$

$$s = 0.0625 \text{ in} \quad l = 1.25 \text{ in}$$

$$A = (6 \text{ in})(4 \text{ in}) = 24 \text{ in}^2 \text{ (cross section area over which } P \text{ acts)}$$

$$\tau = G\gamma \Rightarrow G = \frac{\tau}{\gamma}$$

$$\tau = \frac{V}{A} = \frac{3 \text{ kips}}{24 \text{ in}^2} = \frac{3000 \text{ lb}}{24 \text{ in}^2} = 125 \text{ psi}$$

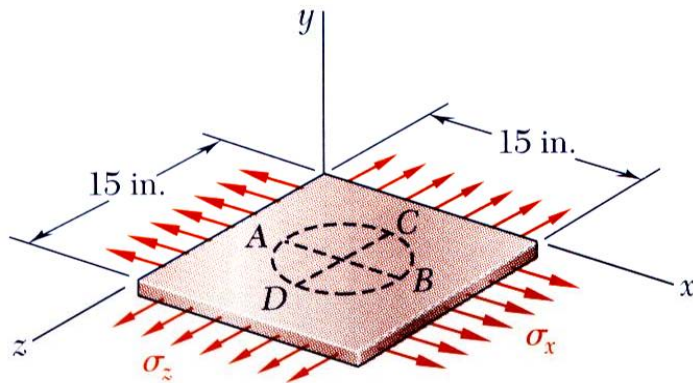
$$\gamma = \frac{s}{l} = \frac{0.0625 \text{ in}}{1.25 \text{ in}} = 0.05$$

$$G = \frac{125 \text{ psi}}{0.05} = 2500 \text{ psi}$$

$$G = 2.5 \text{ ksi}$$



Example Problem



A circle of diameter $d = 9$ in. is scribed on an unstressed aluminum plate of thickness $t = 3/4$ in. Forces acting in the plane of the plate later cause normal stresses $\sigma_x = 12$ ksi and $\sigma_z = 20$ ksi.

For $E = 10 \times 10^6$ psi and $\nu = 1/3$, determine the change in:

- a) the length of diameter AB ,
- b) the length of diameter CD ,
- c) the thickness of the plate, and
- d) the volume of the plate.

(sample problem 2.5 in Beer's book)



Example Problem 2 Solution

- Apply the generalized Hooke's Law to find the three components of normal strain.
- Evaluate the deformation components.

$$\begin{aligned}\varepsilon_x &= +\frac{\sigma_x}{E} - \frac{\nu\sigma_y}{E} - \frac{\nu\sigma_z}{E} \\ &= \frac{1}{10 \times 10^6 \text{ psi}} \left[(12 \text{ ksi}) - 0 - \frac{1}{3}(20 \text{ ksi}) \right] \\ &= +0.533 \times 10^{-3} \text{ in./in.}\end{aligned}$$

$$\begin{aligned}\varepsilon_y &= -\frac{\nu\sigma_x}{E} + \frac{\sigma_y}{E} - \frac{\nu\sigma_z}{E} \\ &= -1.067 \times 10^{-3} \text{ in./in.}\end{aligned}$$

$$\begin{aligned}\varepsilon_z &= -\frac{\nu\sigma_x}{E} - \frac{\nu\sigma_y}{E} + \frac{\sigma_z}{E} \\ &= +1.600 \times 10^{-3} \text{ in./in.}\end{aligned}$$

$$\delta_{B/A} = \varepsilon_x d = (+0.533 \times 10^{-3} \text{ in./in.})(9 \text{ in.})$$

$$\delta_{B/A} = +4.8 \times 10^{-3} \text{ in.}$$

$$\delta_{C/D} = \varepsilon_z d = (+1.600 \times 10^{-3} \text{ in./in.})(9 \text{ in.})$$

$$\delta_{C/D} = +14.4 \times 10^{-3} \text{ in.}$$

$$\delta_t = \varepsilon_y t = (-1.067 \times 10^{-3} \text{ in./in.})(0.75 \text{ in.})$$

$$\delta_t = -0.800 \times 10^{-3} \text{ in.}$$

- Find the change in volume

$$e = \varepsilon_x + \varepsilon_y + \varepsilon_z = 1.067 \times 10^{-3} \text{ in}^3/\text{in}^3$$

$$\Delta V = eV = 1.067 \times 10^{-3} (15 \times 15 \times 0.75) \text{ in}^3$$

$$\Delta V = +0.187 \text{ in}^3$$

